

Greenstick innovations for dredging

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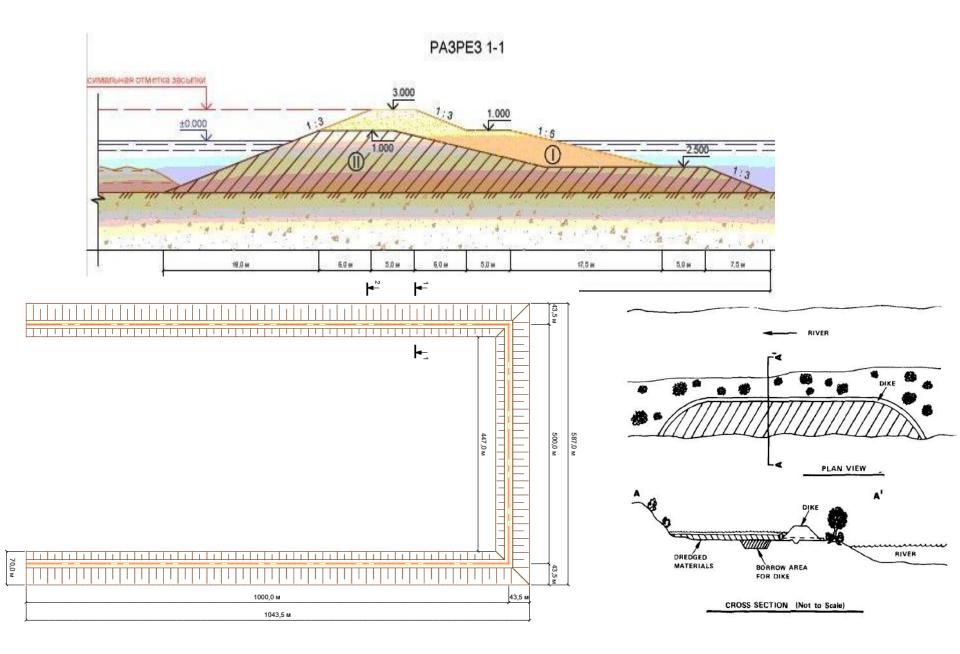


Artificial territories of dredged soils

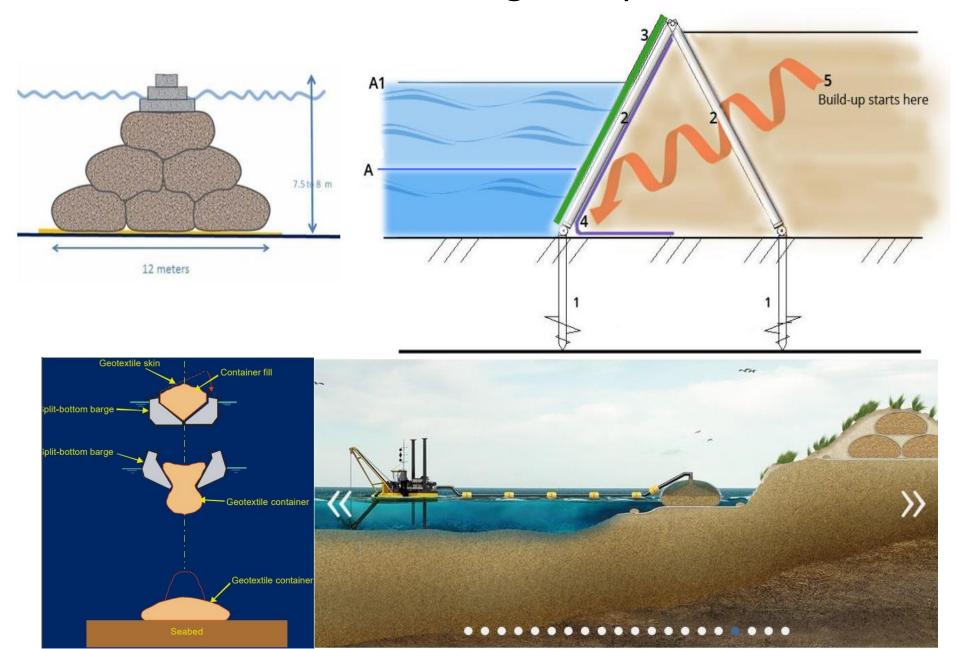


Artificial territory of dredged soils. Rostoc, Drigge, Germany

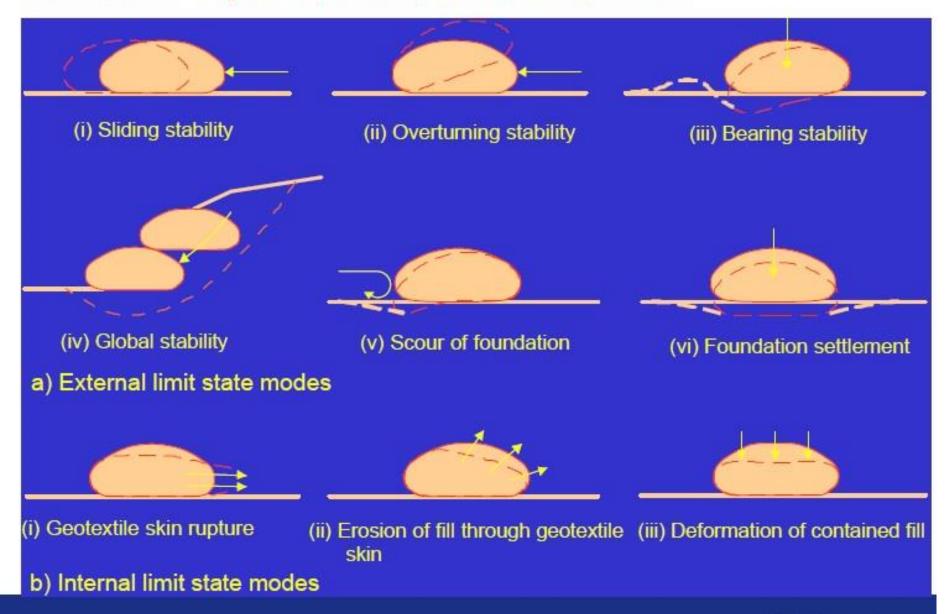
Dams for dredged deposits



Dams for dredged deposits



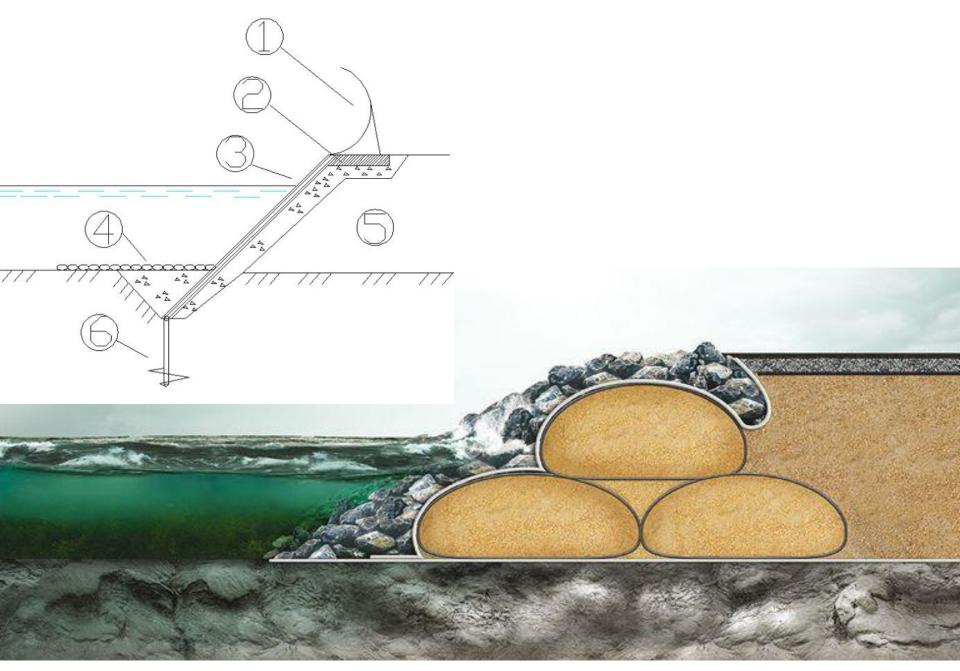
Geotube® systems: limit state modes



Steel vertical "fence" structure

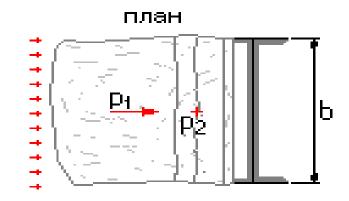


Greenstick Sea Wall

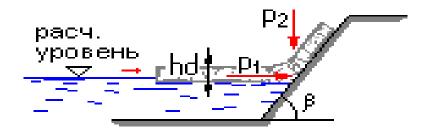


Ice loads calculations for inclined wall









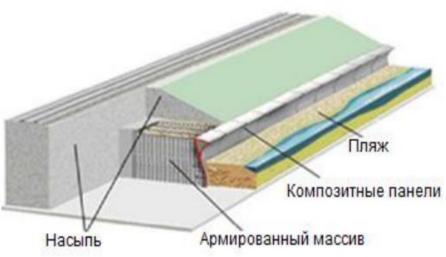


Ice loads calculations for inclined wall

Characteristics		Symbol	Quantity	Measure	
Ice –air temperature, C	Ice –air temperature, C		-5	°C	
Ice width		Hd	0.1	m	
Max. square of ice field insurance	d, 1%	Al	100	m2	
Speed of ice field		vl	0.15	m/sec	
Characteristisc	Symb	ol	Quantity	Measure	
Vertical force from ice movement	P1		108	кН	
Horizontal force from temperature extension of ice	P2		108	кН	

FRP: Apatech Sea Wall and Australian Breackwater









Infussion and pultrussion

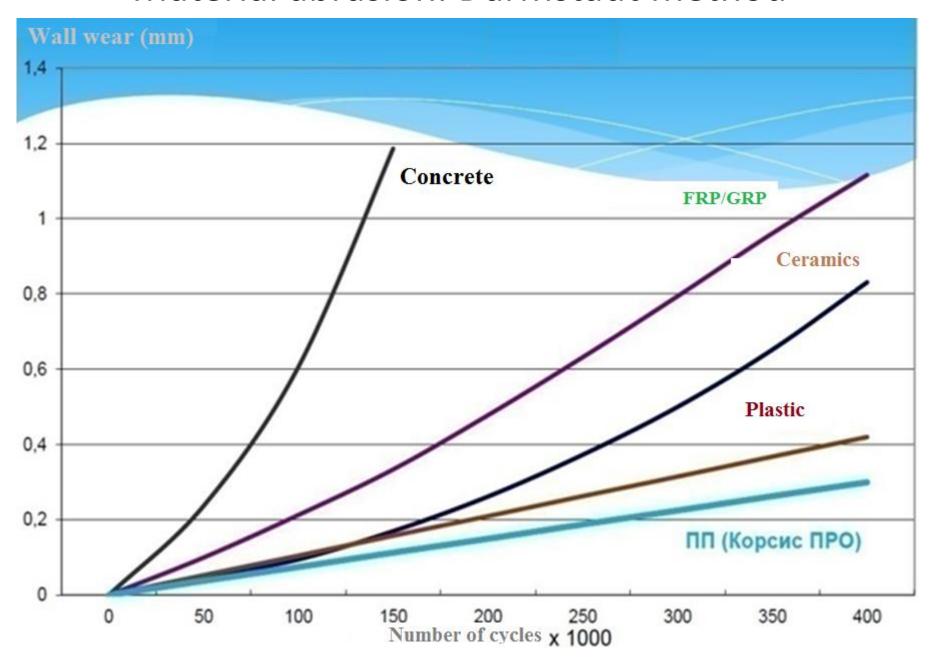




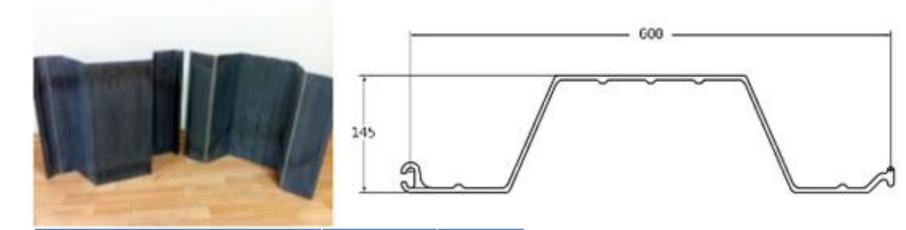




Material abrasion. Darmstadt method



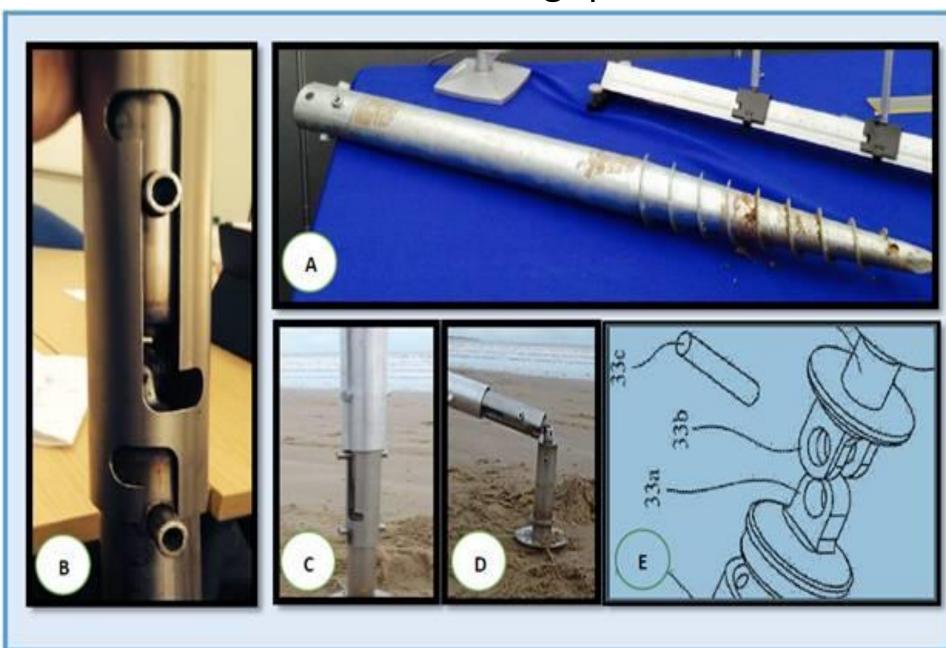
Physical characteristics of FRP SHK-150 sheet pile



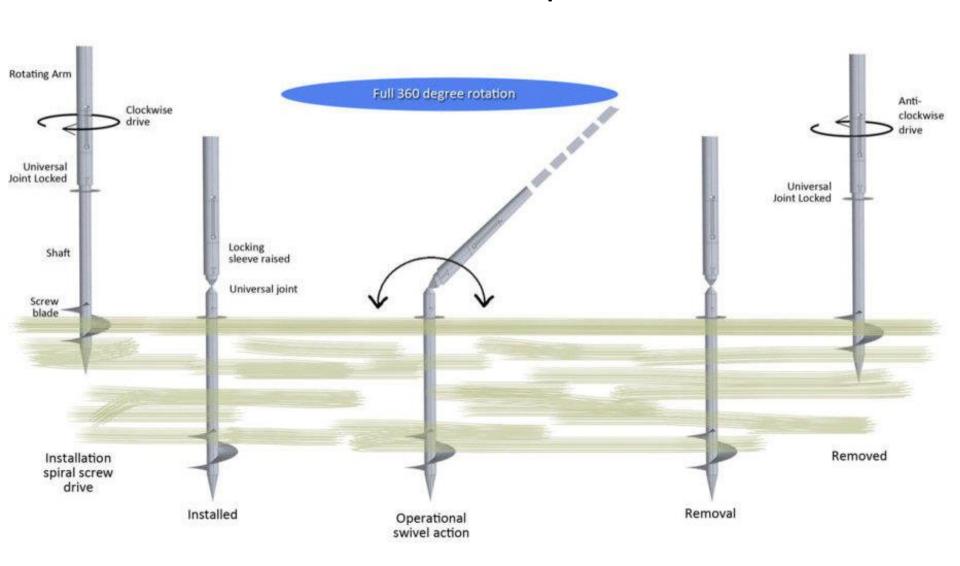
Characteteristics	Measure	Value
Allowable moment (M)	kN-m/m	237
Inertion moment (I_y)	c m^4 /m	17885
Inertion moment (I_x)	cm^4/m	1684
Resistant moment (W_x)	cm^3/m	210
Strength limit (R)	MPa	1126
Width	mm	600
Depth	mm	145
Thickness	mm	5

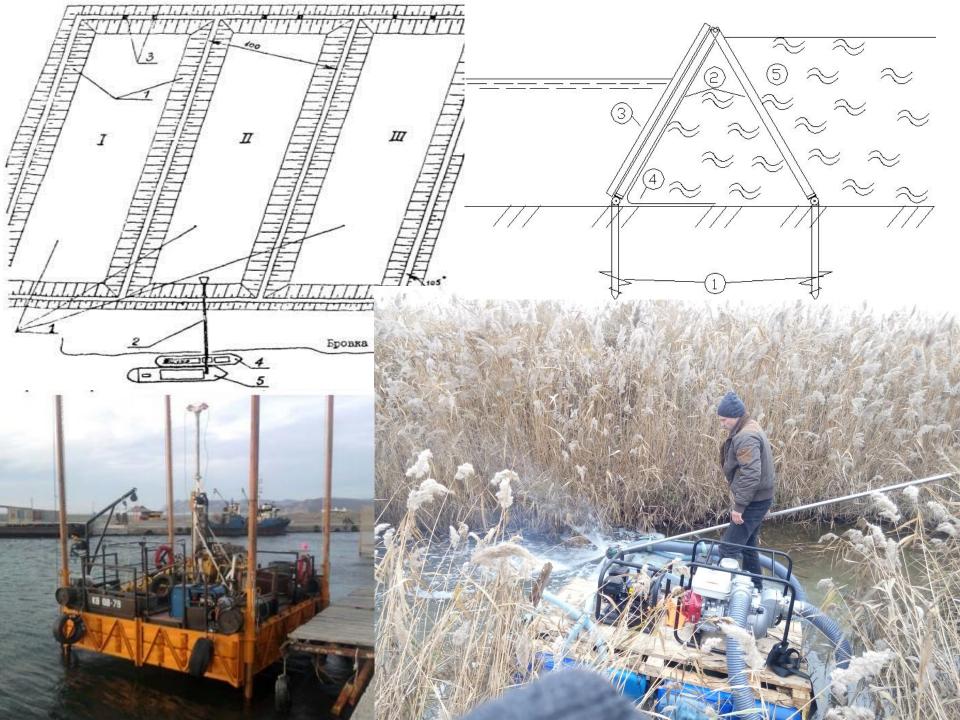


Greenstick grip

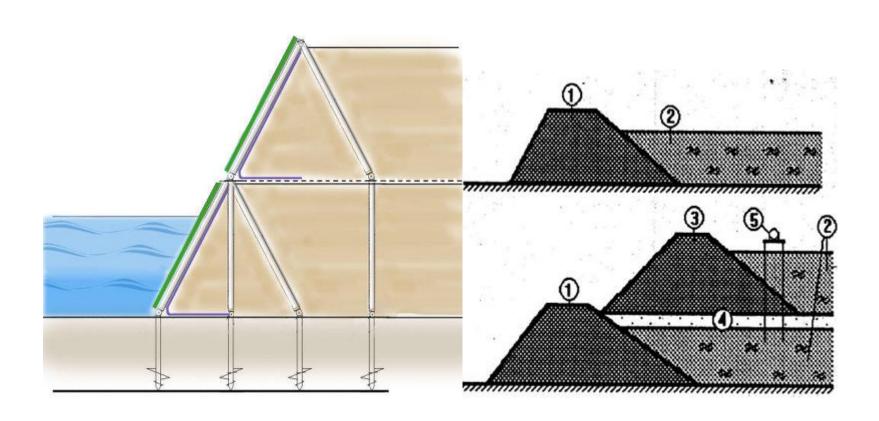


Greenstick pile

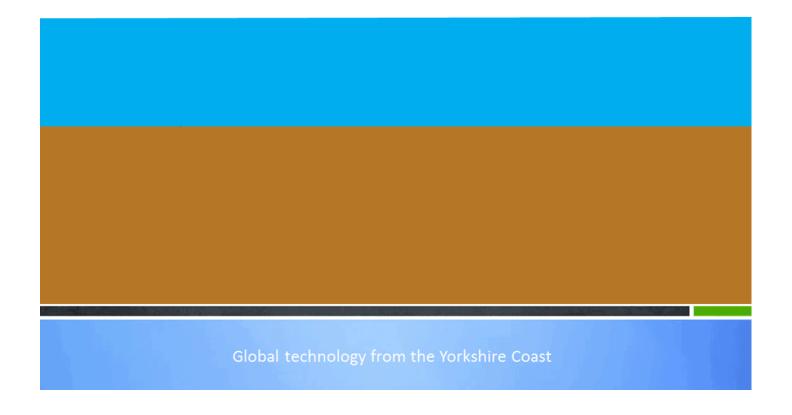




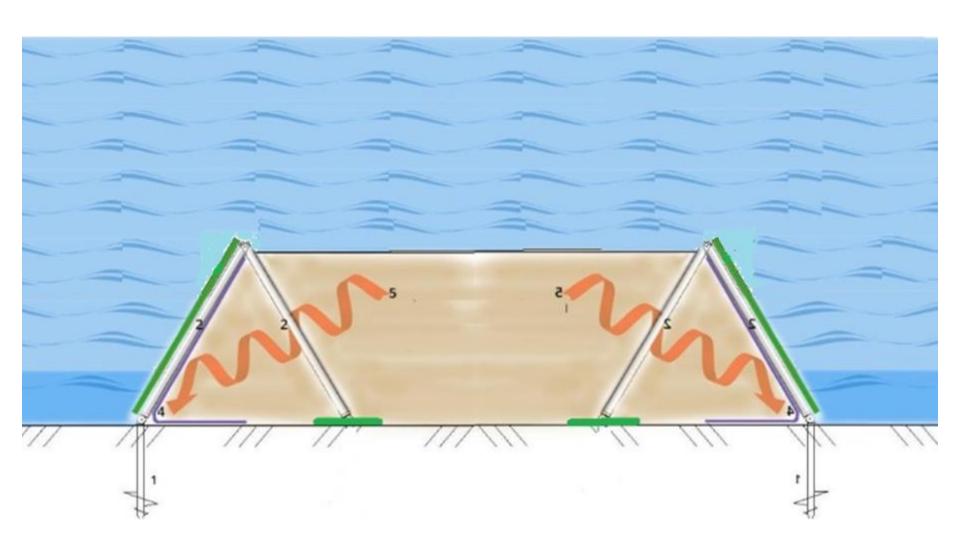
Greenstick wall installation for 2nd dredged layer



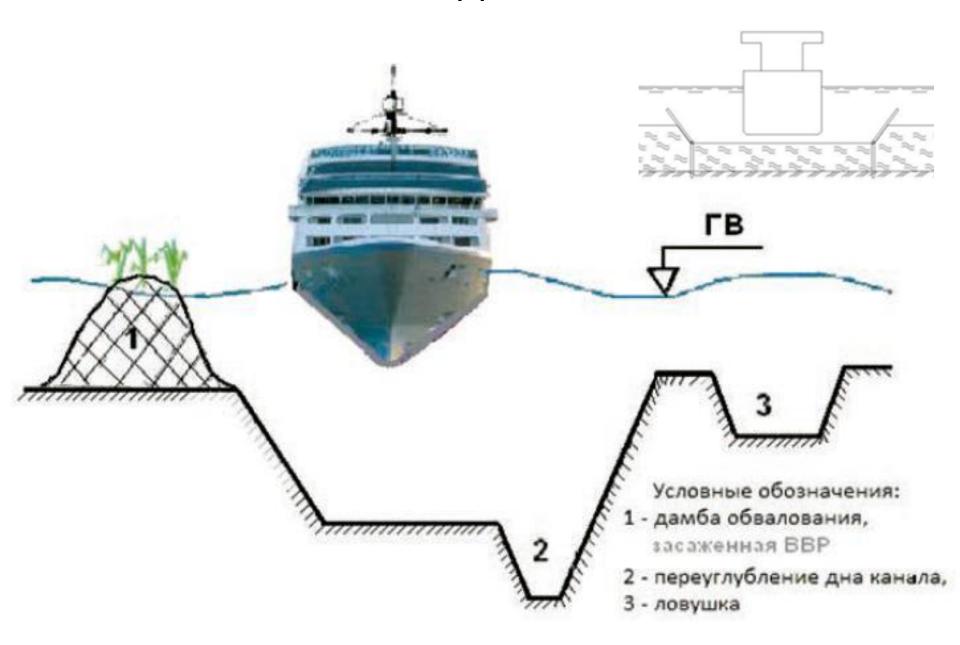
Technology of installation of Greenstick Walls



Decisions for underwater dumping deposits



Decisions for approach channels



Tests of Walls in Waves flume

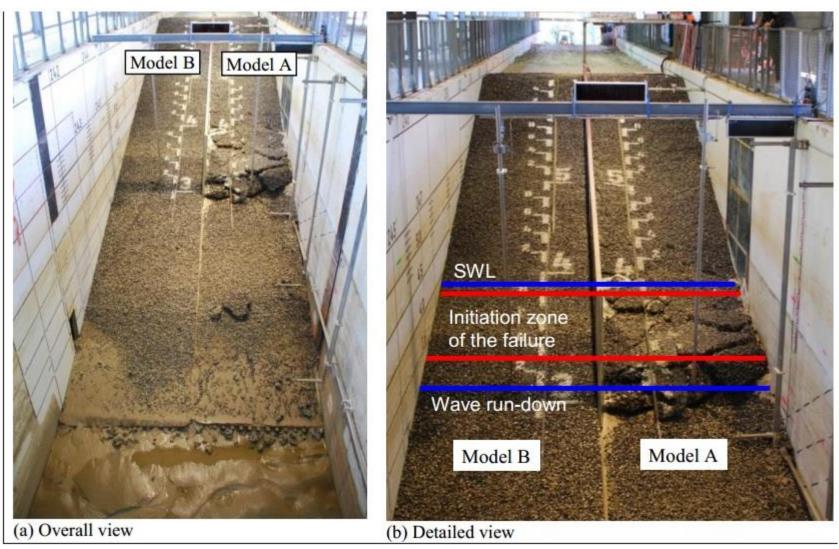
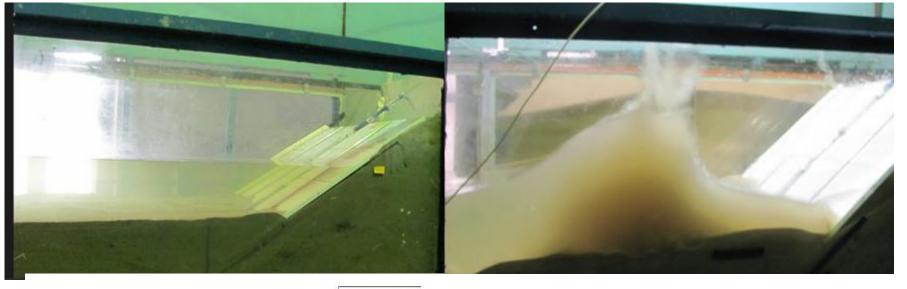
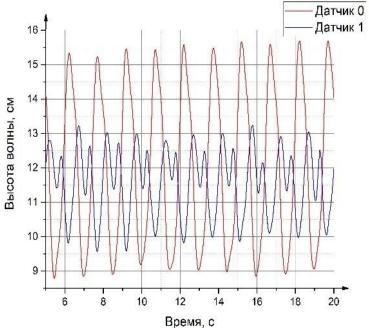


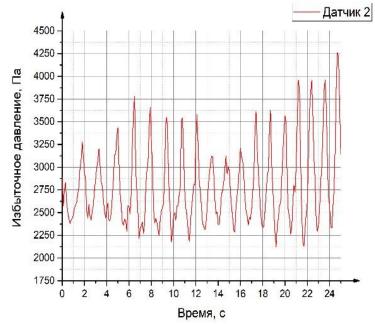
Figure 8. Extent of damage for revetment Model A after regular wave test 09051802 with H =1.3m, T = 5.0s and h =3.9m

Tests of Greenstick Wall in Waves flume of

http://hydromech.org.ua/en/



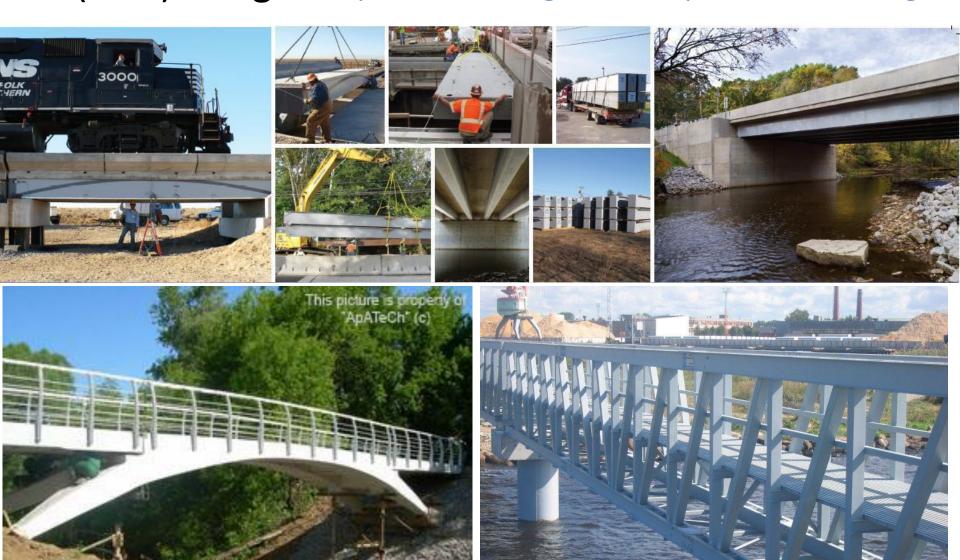




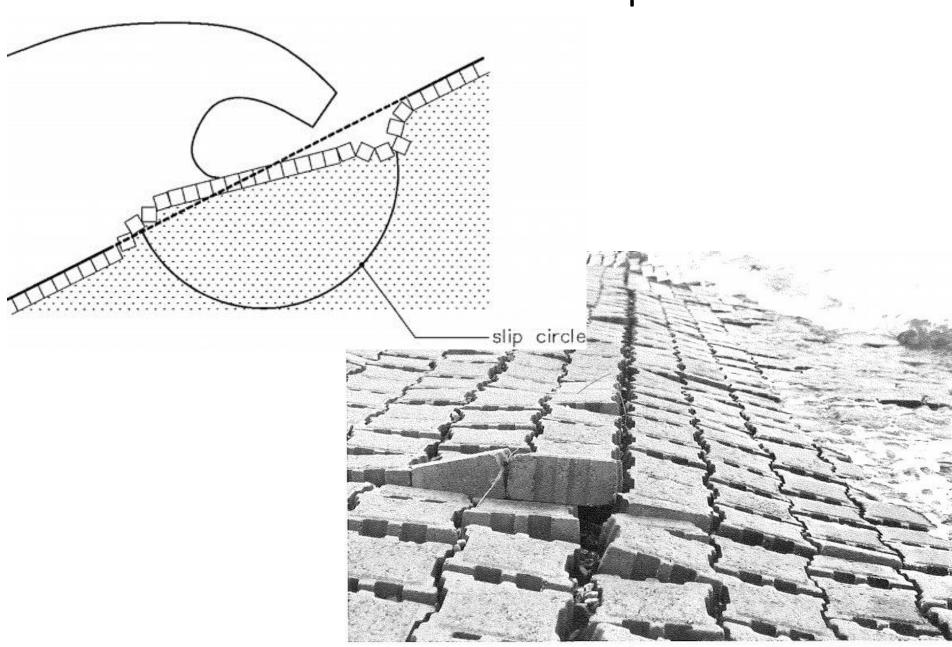
Waves flume's tests results

Expe- riment №	Gauge	Period, sec	Medium waves height, sm	min waves height, sm	max waves height, sm	Reflec- tion rate	Waves height, m	Waves spead, m/sec
	0	0,68	4,484	3,361	6,07	0,399		
14_11_otr	1	0,619	4,506	3,058	6,57	0,404	0,373	0,274
_1	2	1,227	7,248	5,787	9,839	0,211	(0,658)	(1,013)
	3	1,237	7,705	5,106	10,756	0,438		
	0	1,897	3,388	2,021	3,999	0,156		1,938
23 11 1	1	1,107	1,189	0,335	1.757	0,744	3,784	
23_11_1	2	2,008	4,653	3,739	5,386	0,176	(5,949)	(3,046)
	3	1,402	1,75	0,135	3,069	0,915		
	0	1,836	4,574	2,365	5,651	0,21		
23 11 2	1	1,064	1,273	0,239	2,538	0,833	3,705	1,97
23_11_2	2	1,925	6,019	5,063	6,456	0,171	(5,517)	(2,934)
	3	1,73	2,418	1,659	2,843	0,909		

FRP(HCB) bridges http://innovativeglobal.net/products/hc-bridge

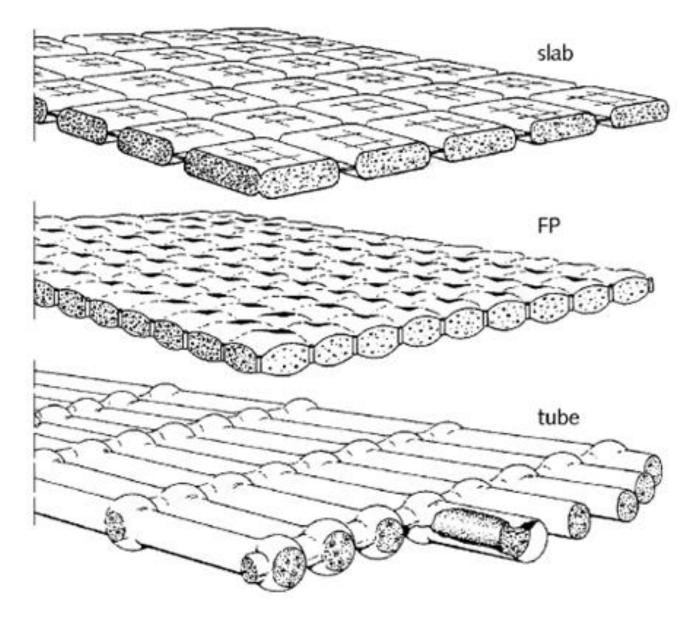


The distortion of an S-profile

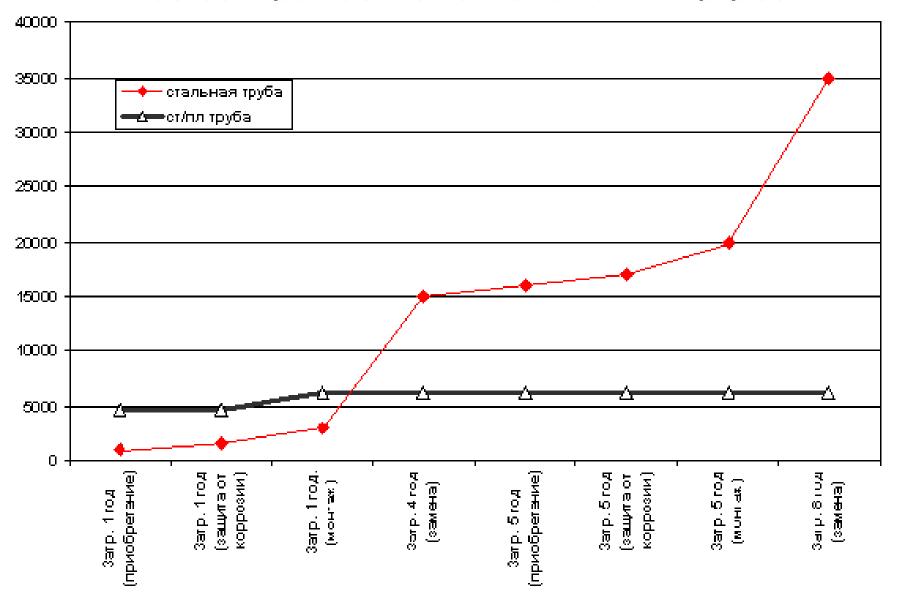


The turned-up corner of a mattress - test section near Lelystad.

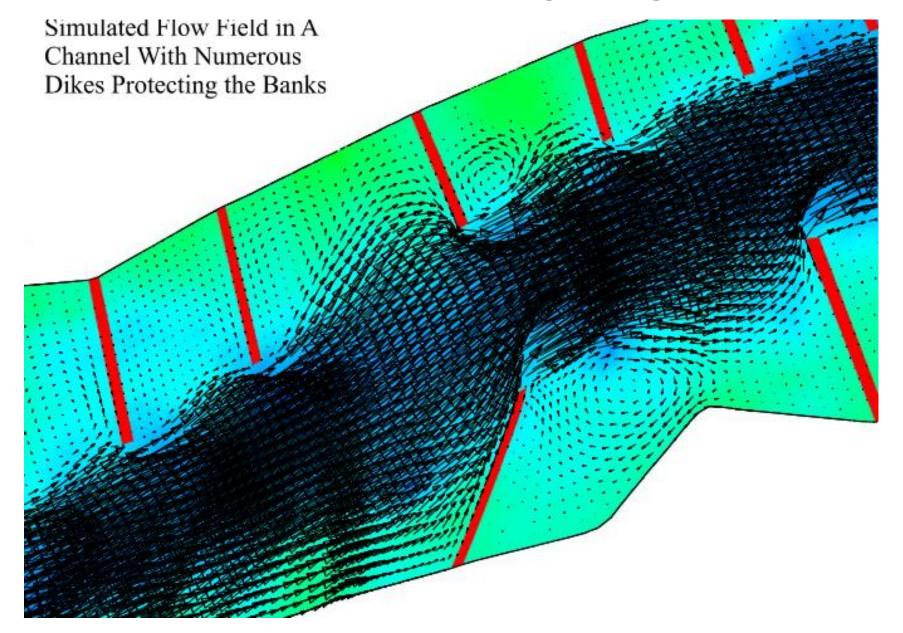
Concrete-filled mattresses



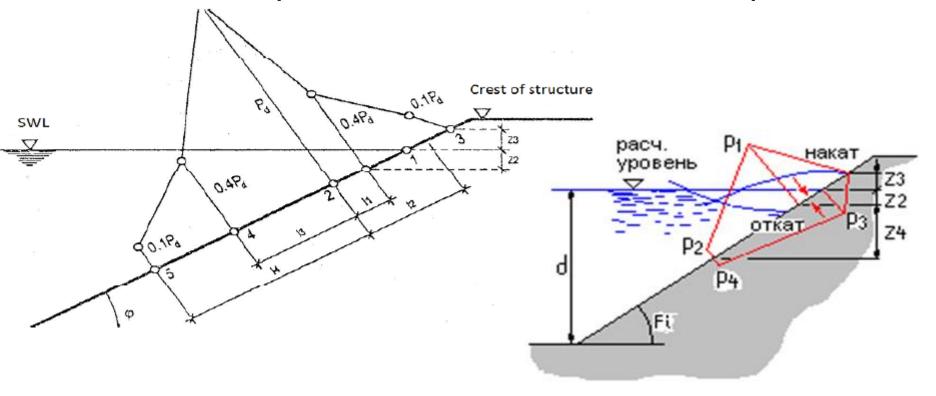
Price comparison of steel and FRP pipes



Decisions for underwater dums guiding the stream

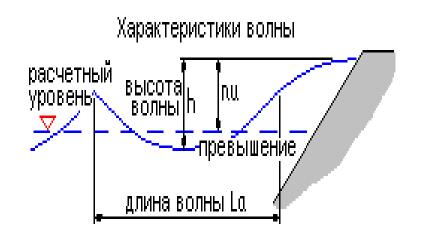


Wave pressure distribution on slope



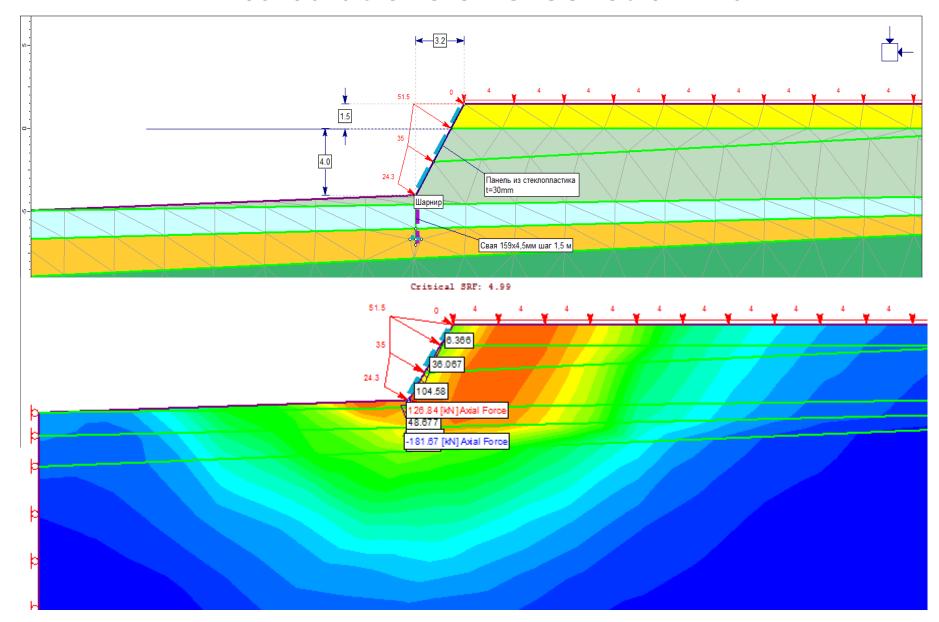
Scheme characteristics	Symbol	Quantity	Measure
Wind speed, 10m hight		15	m\sec
Bottom depth	(d)	2	m
Slope angle	(Fi)	44	0
Plates size	(bf)	30	m

Waves loads calculations for Greenstick inclined wall

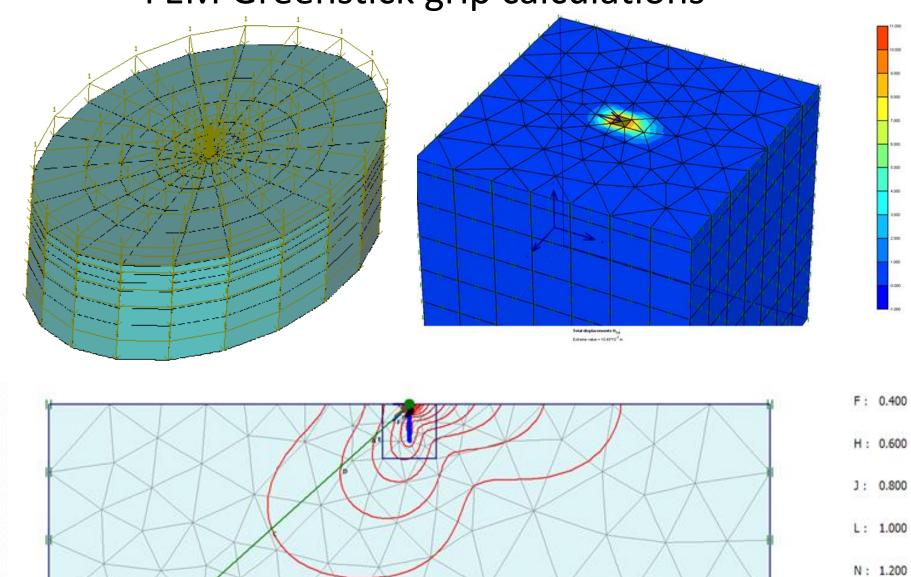


Waves characteristics	Symbol	Quantity	Measure
Medium waves height	(h_)	2.25	m
Medium waves length	(La_)	33.88	m
Medium waves period	(T_)	7.11	sec
First waves collapse depth	(dcr)	1.68	m
last waves collapse depth	(dcru)	0	m
Calculated waves height in front of wall	(hr)	4.26	m
Exceedance of max. wave above of calculated level	(nu)	3.2	m

FEM calculations of Greenstick Wall



FEM Greenstick grip calculations



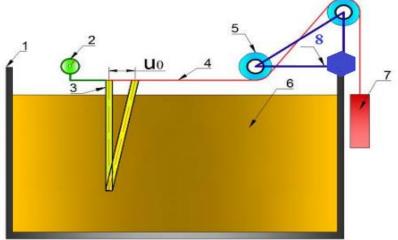
0.00

-5.00

Tests of Greenstick grip in soil tray of BLASDARI



www.blasdari.com



Pile`s	Anchorage aility, kg, for angle of inclined uplift force								
type	Hori-	20	30	45	60	Vertical			
	zontal								
"Krinner	42,40	57,49	26,34,	33,42,3	48	32			
" pile			32	5					
Screw	40	58,62	55,72	55,62	57,65	83,80,72			
pile									

Vertical uplift capacity of "Kriner" screw pile

$$Fd = \sin\beta \times \Sigma[\gamma c \times (a1 \times C1 + a2 \times P \times hi) \times A] + [\gamma c \times U \times fi \times (Hi - d)]$$

- Fd (tons) = Vertical uplift capacity
- β (°)=Angle of Thread
- hi(m) = The depth of each i-th thread.
- A(m2) = The projection of the working area of the blade.
- U(m) = Perimeter of pile shaft
- Hi (m)= Shaft length, measured from ground surface to first thread
- d(m) = Diameter of the pile shaft
- γc (dimentionless)= Coefficient of working conditions, for compressing, tensile and alternating loads
- a1 & a2 (dimentionless)= Constants dependent upon φ
- φ (°) = The estimated value of the angle of internal friction of soil in the work area
- C1 (tonsm2/) = The estimated value of the specific adhesion of soil in the work area
- P(tonsm3/) = Unit weight of the soil, determined through Appendix A.
- *e* (*dimentionless*) = Porosity, the results of engineering-geological surveys
- fi(tonsm2/) = Calculated resistance of the soil on the screw piles
- 4.3, by taking the median value for all layers within the depth of the pile (above the thread).
- *IL* (*dimentionless*) = Flow index

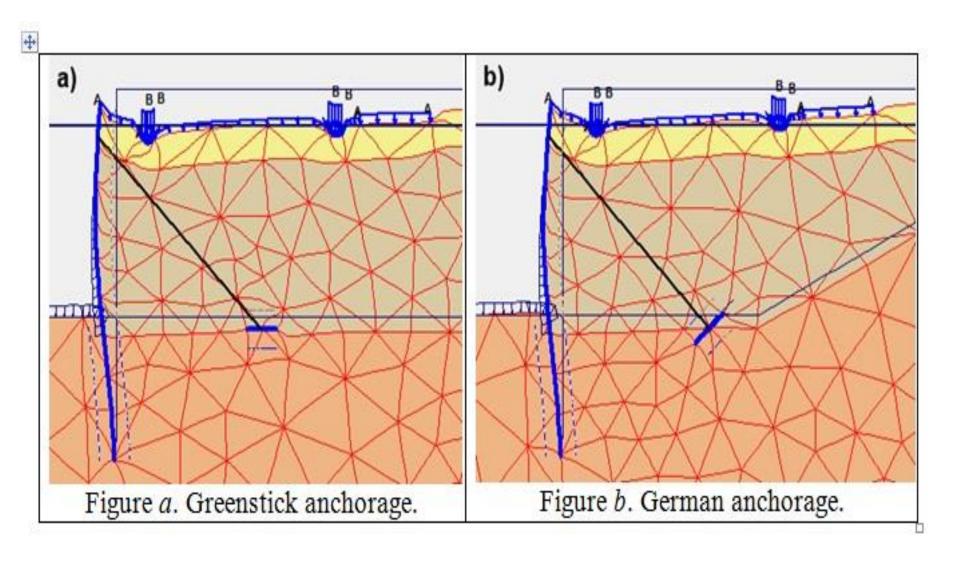
Soil type		Vertical bearing capacity (tons) of Greenstick screw pile of size 900x74, depending on the porosity (inverse of density) of the soil. For compressive loading (top), tensile loading (middle) and alternating loads (bottom).								
		0.45	0.55	0.65	0.75	0.88	0.95	1.05		
Large	Slightly wet	1.116	0.947	20	2	2	14	140		
sand	CONTRACTOR AND CONTRACTOR	0.837	0.710	-	7.		-	07.0		
particles		0.418	0.355	-	-	-	-	-		
Wet	0.976	0.829	-	2	-	12	920			
	0.697	0.592	-	73	-		·			
		0.279	0.237	-	¥	-	_	-		
	Saturated	0.837	0.710	-	i e	-	-	-		
		0.558	0.474	-	-	-	-	10 0 10		
		0.139	0.118	_	2	.=	2	(¥0)		
Average	Slightly wet	1.218	1.049	0.947	-	-		65.0		
sand		0.914	0.787	0.710	-	-	-			
particles		0.457	0.394	0.355	23	<u>=</u>	_	120		
**************************************	Wet	1.066	0.918	0.829	-	-	-	878		
	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	0.761	0.656	0.592	-	<u>_</u>	-	(m)		
		0.305	0.262	0.237	70	2	1/2	0260		
	Saturated	0.914	0.787	0.710	1-	-	-	19.0		
		0.609	0.525	0.474	2	2	2	-		
		0.152	0.131	0.118	-		-			
Small	Slightly wet	1.449	1.244	0.734		-	-			
sand		1.087	0.933	0.543	2	2	2	92//		
particles		0.543	0.467	0.271		-		(-):		
Ži	Wet	1.268	1.089	0.633	-	-	-	-		
	20.575	0.906	0.778	0.452	2	2	_	-		
		0.362	0.311	0.181	-	-	-	5 82		

1	Saturated	1.087	0.933	0.543				-
	0.2398729375 1.7747.094722.4	0.725	0.622	0.362	-	-	-	-
		0.181	0.156	0.090	-	-	-	-
Silty	Slightly wet	1.634	1.429	0.639	0.305	-	-	-
sands		1.225	1.071	0.480	0.229	-	-	-
		0.613	0.536	0.240	0.114	_	2	-
	Wet	1.429	1.250	0.559	0.267	-	-	-
		1.021	0.893	0.400	0.190	-	-	: -
		0.408	0.357	0.160	0.076	2	2	<u>12</u>
	Saturated	1.225	1.071	0.480	0.229	-	-	-
		0.817	0.714	0.320	0.152	2	2	-
		0.204	0.179	0.080	0.038	-	-	-
Sandy	Solid State	1.750	1.341	0.931	0.587	-	-	
Loam	$0 \le I_L \le 0.25$	1.313	1.005	0.699	0.440	2	2	22
1001-506/1811-10	335-40-35-40-40-33-33-35-35-35-35-35-35-35-35-35-35-35-	0.656	0.503	0.349	0.220	-	-	-
	Plastic State	1.050	0.721	0.497	0.331	0.217	-	
	$0.25 < I_L \le$	0.750	0.515	0.355	0.236	0.155	2	-
	0.75	0.300	0.206	0.142	0.094	0.062		-
	Flowable State	0.917	0.601	0.413	0.270	0.173	-	=
	$0.75 < I_L$	0.612	0.401	0.275	0.180	0.115	-	-
	376	0.153	0.100	0.069	0.045	0.029	-	
Loam	Solid State	2.031	1.489	1.101	0.853	0.705	0.511	12
	$0 \le I_L \le 0.25$	1.523	1.117	0.826	0.640	0.529	0.383	-
		1.269	0.930	0.688	0.533	0.441	0.319	-
	Plastic State	1.324	1.076	0.820	0.634	0.433	0.323	-
	$0.25 < I_L \le$	0.993	0.807	0.615	0.476	0.325	0.242	-
	0.75	0.662	0.538	0.410	0.317	0.217	0.161	-
	Flowable State	0.50	253	0.476	0.363	0.280	0.208	0.178
	$0.75 < I_L$		*	0.340	0.259	0.200	0.149	0.127
	Section 1 Control	121	-	0.136	0.104	0.080	0.060	0.051
Clay	Solid State		1.920	1.458	1.089	0.874	0.722	0.558
- 60	$0 \le I_L \le 0.25$	5.43	1.440	1.093	0.816	0.655	0.542	0.419
		•	1.200	0.911	0.680	0.546	0.451	0.349
	Plastic State	-		1.029	0.876	0.728	0.550	0.462
	$0.25 < I_L \le$		-	0.772	0.657	0.546	0.413	0.347
	0.75	-	-	0.515	0.438	0.364	0.275	0.231
	Flowable State	148	-	0.594	0.523	0.440	0.407	0.360
	$0.75 < I_L$		-	0.424	0.373	0.314	0.291	0.257
				0.170	0.149	0.126	0.116	0.103

Installation Torque – Results for "Krinner" screw pile

Soil Type (Sand Only)		Installation Torque (kN.m) of Greenstick screw pile of size 900x74, depending on the porosity (inverse of density) of the soil. Expressed as a range 10-15% greater than that calculated using equation 4.2, for the reasons explains at the end of section 4.4.								
		0.45	0.55	0.65	0.75	0.85	0.95	1.05		
Large sand	Slightly wet	0.136 0.143	0.086			-	-	-		
particles	Wet	0.119 0.125	0.076 0.079	•	•	•	-			
Saturate	Saturated	0.102 0.107	0.065 0.068	×		-	-	-		
sand	Slightly wet	0.113 0.118	0.076 0.080	0.048 0.051	100	•				
	Wet	0.099 0.104	0.067 0.070	0.042 0.044	•	-	-	-		
	Saturated	0.085	0.053 0.055	0.036 0.038		-	-	-		
Small sand	Slightly wet	0.094	0.075 0.079	0.051 0.053		-		-		
particles	Wet	0.083 0.086	0.066 0.069	0.044 0.046	•	2	=	•		
	Saturated	0.071 0.074	0.057 0.059	0.038 0.040	•	-	-	-		
Silty sands	Slightly wet	0.083 0.087	0.067 0.070	0.044 0.046	0.030 0.031	-	-			
	Wet	0.073 0.076	0.058 0.061	0.038 0.040	0.026 0.027	28	-	-		
	Saturated	0.063	0.050 0.052	0.033 0.034	0.023 0.024	-	-	-		

Greenstick anchorage (screw part not showed)



Efficiency Benefits of Greenstick piles

- Vertical deployment:
- No requirement to drill at angle
- Smaller, lighter weight Greenstick may be used
- Smaller drilling devices may be used
- Speedier deployment
- Rotating arm allows more flexibility to link to structure
- Stability of structure:
- Beneficial in shifting patterns of deposition of bed material
- Potential reduction in scour protection methods
- Angled Greenstick provides solid foundation
- Substantially smaller dimensions required than conventional piles:
- Lighter piles can be transported more easily
- Energy efficiencies
- Re-deployment
- Re-use rather than new manufacture
- Temporary structures often gain quicker Planning agreement

Environmental Benefits of Greenstick piles

- Less deoxygenation of water helps protect sea life
- Screw piles rather than pile driven:
- Less impact on the sea-bed
- Reduced impact on sea mammals habitat
- Removable, allowing the environment to recover
- Damage limitation, particularly in flood defence projects
- Smaller vessels used for deployment
- Recyclable

Financial Benefits of Greenstick piles

- Production costs:
- Simple design, few complex parts, variety of materials may be used
- Lighter weight, requiring less raw material
- Transportation costs:
- Lighter weight product
- Deployment costs:
- Smaller vessel required
- Easier to deploy than conventional piles
- Wastage costs:
- Greenstick can be easily reclaimed and re-used
- Environmental costs:
- Less intrusive to marine environment

Thank you for attention

